### 1.12 I-526 DHVs

**Table 11** and **Table 12** show the existing and projected future 2030 and 2050 DHVs for I-526, east of Long Point Road at ATR 90. The future DHVs were estimated using the growth factors presented in the Growth Rate section of this technical memorandum. The PM peak hour DHV was the 30<sup>th</sup> highest hour in 2019, which was Thursday October 3. The AM peak hour peak hour DHV was the 146<sup>th</sup> highest hour in 2019, which was Monday September 9.

AM Peak Hour 7:00 to 8:00 AM YEAR Eastbound Westbound Total 2022 2,587 2,783 5,370 2030 2,950 3,170 6,120 2050 4,040 4,340 8,380

Table 11: I-526 AM Peak Hour DHV Mainline Directional Volumes

Table 12: I-526 PM Peak Hour DHV Mainline Directional Volumes

PM Peak Hour 5:00 to 6:00 PM					
YEAR	Eastbound	Westbound	Total		
2022	3,386	2,364	5,750		
2030	3,860	2,690	6,550		
2050	5,280	3,690	8,970		

## 1.12.1 Interchange DHV Factors

February 2022 traffic volumes along Long Point Road and Seven Farms Drive were converted to DHVs using the discussed COVID-19 factor and factors developed to estimate the 30th highest hour (PM peak) and 146th highest hour (AM peak). **Table 13** display the equivalent

dates in February 2019 to compare average total I-526 traffic volumes to the 30th and 146th highest hour of ATR 90. A 5% factor was determined to increase the February counts to be as high as an

	AM	PM
COVID Factor	10%	0%
DHV Factor	5%	5%

October count pre-COVID. These 2022 total intersection volumes were independently compared to the original, unfactored 2017 TMC of the study intersections included in the 2017 LLC I-526 EAST PEL study. In all cases, the total hourly entering intersection volume was lower in 2022 than the previous approved turning movement volume totals based on 2017 counts as shown in Table 13. The yellow highlighted value indicates the larger total volume.

Table 13: Equivalent February 2019 compared to Design Hour Volumes

2019		2019		
Equivalent Dates	5 6 PM 2 Way Count	Equivalent Dates	7-8 AM 2-Way Count	
6-Feb	5,541	6-Feb	5,089	
26-Feb	5,469	26-Feb	5,060	
27-Feb	5,389	27-Feb	5,182	
Average 5,466		Average	5,110	
DHVfactor	1.05	DHVfactor	1.05	
30th Highest Hour		146th Highest Hour		
10/3/2019	5,750	9/9/2019	5,370	

Table 14: 2022 Factored DHV vs. 2017 Unfactored TMC at Long Point Road and Daniel Island Interchanges

Long Point Road Intersection at:	Time Period	Data Set	<b>Hourly Totals</b>	Amount less
Belle Point Dr	07:00-08:00 AM	2022 DHV with factors	3046	97%
		2017 TMC unfactored	3126	
Belle Point Dr	05:00-06:00 PM	2022 DHV with factors	3209	91%
		2017 TMC unfactored	3515	
Belle Hall Pkwy	07:00-08:00 AM	2022 DHV with factors	3258	98%
		2017 TMC unfactored	3320	
Belle Hall Pkwy	05:00-06:00 PM	2022 DHV with factors	3456	90%
		2017 TMC unfactored	3831	
I-526 NB Ramps	07:00-08:00 AM	2022 DHV with factors	3958	97%
		2017 TMC unfactored	4094	
I-526 NB Ramps	05:00-06:00 PM	2022 DHV with factors	4134	94%
		2017 TMC unfactored	4378	
I-526 SB Ramps	07:00-08:00 AM	2022 DHV with factors	3266	98%
		2017 TMC unfactored	3325	
I-526 SB Ramps	05:00-06:00 PM	2022 DHV with factors	3576	91%
		2017 TMC unfactored	3948	
Wando Park Blvd/Hobcaw Bluff Dr	07:00-08:00 AM	2022 DHV with factors	2078	99%
		2017 TMC unfactored	2108	
Wando Park Blvd/Hobcaw Bluff Dr	05:00-06:00 PM	2022 DHV with factors	2185	88%
		2017 TMC unfactored	2487	
Shipping Ln/Hidden Blvd	07:00-08:00 AM	2022 DHV with factors	1192	0%
		2017 TMC unfactored	0	
Shipping Ln/Hidden Blvd	05:00-06:00 PM	2022 DHV with factors	1161	0%
		2017 TMC unfactored	0	
Wando Ln	07:00-08:00 AM	2022 DHV with factors	671	0%
		2017 TMC unfactored	0	
Wando Ln	05:00-06:00 PM	2022 DHV with factors	560	0%
		2017 TMC unfactored	0	
<b>River Landing Drive Intersection at:</b>	Time Period	Data Set	Hourly Totals	<b>Amount less</b>
Fairchild St	07:00-08:00 AM	2022 DHV with factors	1796	84%
		2017 TMC unfactored	2133	
Fairchild St	05:00-06:00 PM	2022 DHV with factors	1412	71%
		2017 TMC unfactored	1984	
Island Park Dr	07:00-08:00 AM	2022 DHV with factors	1311	87%
		2017 TMC unfactored	1509	
Island Park Dr	05:00-06:00 PM	2022 DHV with factors	1112	74%
		2017 TMC unfactored	1493	
Seven Farms Dr	07:00-08:00 AM	2022 DHV with factors	1857	94%
		2017 TMC unfactored	1968	
Seven Farms Dr	05:00-06:00 PM	2022 DHV with factors	1820	95%
		2017 TMC unfactored	1906	

A review of the individual turning movements measured in 2022 indicated that some changes have occurred that yielded a higher vehicle count in 2022 than the previous 2017 data set. These changes were attributed to a new Chick-fil-A restaurant on Belle Point Drive and housing occupancy increases in the area. Resulting calculations displaying the locations where 2022 turning movement volumes were used with the 2017 previously approved data set are included in the **APPENDIX**.

#### 1.12.2 Peak Hour Factors

A comparison of peak hour factors (PHF) between 2017 and 2022 turning movements were conducted. In most cases, the 2017 PHF was higher or equal to the 2022 measures PHF. **Figure 18** shows a display of the PHFs by intersection location.

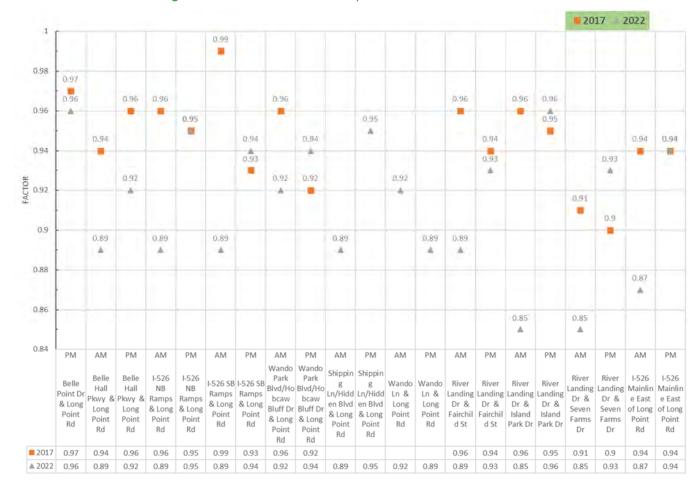


Figure 18: Peak Hour Factor Comparison - 2017 vs. 2022

# 2.0 SUMMARY AND RECOMMENDATIONS

This Design Hour Volume technical memorandum has presented a methodology to independently develop factors to apply to the 2022 traffic data to the 30th highest hour of a typical year pre-COVID-19 operation using best practices to compare the previously accepted and approved 2017 traffic data used in the 2017 I-526 LCC EAST PEL study. Despite best efforts, 2022 design hour volumes on Long Point Road and Seven Farms Drive interchanges and intersections are less than 2017 design hour volumes. Long Point Road and Seven Farms interchanges are not demonstrating the same characteristics as I-526 mainline. The impact of COVID-19 on I-526 continues to occur in the AM peak but not as noticeable in the PM peak hour. A conclusion was determined that the 2022 volumes, even factored, are smaller than the 2017 DHV's. A recommendation of this design hour volume technical memorandum to develop 2022 DHVs is to:

- Use 2017 LCC DHVs with a few modifications of higher 2022 DHVs
- Use growth rate as per LCC EAST PEL analysis output
  - 1.6% annual growth on I-526 freeway and ramps
  - 1.0% on Long Point Rd north of I-526
  - 1.2% on Long Point Rd south of I-526
- Adopt design years
  - 2022,
  - 2030, and
  - 2050
- Use 2017 truck percentages for Long Point Road but increase based on Wando Port growth projections provided
- Use 2017 PHFs

## 2.1 2022 DHVs

Figure 19 displays the 2022 design hour volume traffic volumes for the Seven Farms Drive interchange and intersections. Figure 20 displays the 2022 design hour volumes for the Long Point Road interchange and intersections. Figure 21 displays the 2022 design hour volume traffic volumes for the Hungryneck Boulevard interchange.

As a final check of the proposed 2022 Long Point Road ramp DHVs, an analysis of K-factors (DHV/2018) AADT) was undertaken with the expectation that they should generally be between 0.08 and 0.11. All the 2022 K-factors were within the anticipated range except:

- The northwest quadrant on-ramp from Long Point Road to westbound I-526 where the AM peak hour K-factor is 0.19. The 2018 AADT was 7,700 vpd.
- The southeast quadrant loop on-ramp from Long Point Road to westbound I-526 where the AM peak hour K-factor is 0.07 and the PM K-factor is 0.06. The 2018 AADT was 7,500 vpd.
- The on-slip ramp from Long Point Road to eastbound I-526 where the AM peak hour K-factor is 0.06. The 2018 AADT was 4,600 vpd.

Each of these links facilitate one-way movements which often fall outside the standard 0.08 to 0.11 range that is typical for a bidirectional roadway. Reasons can include an offsetting opposite movement at a different ramp that has a high directional pairing with the subject ramp. Although somewhat lower than typical, the 0.06 and 0.07 K-factors were judged acceptable. The 0.19 K-factor required more investigation. In 2017, the on-ramp from Long Point Road to westbound I-526 had a measured AM peak hour volume of 1,231 vph, producing a K-factor of 0.16. The value of 1,231 vph was converted to a 2017 DHV of 1,385 vph with balancing and smoothing techniques applied, resulting in a K-factor of 0.18. From this analysis, it was concluded that the 2022 DHV is reasonable because of either a large surge of AM peak hour traffic or a low AADT.

# 2.2 2030 DHVs, TRUCK, TOTAL

Traffic volumes in 2022 were grown with compounded growth rates based on the CHATs TDM to develop 2030 traffic volumes. Figure 22 displays the 2030 DHV's for the Seven Farms Drive interchange and intersections. Figure 23 displays the 2030 DHV's for the Long Point Road interchange and intersections. The Long Point Road Interchange is unique because of the substantial number of tractor trailer trucks compared to single unit trucks. Growth of the Wando Port terminal will be incorporated into 2030 Long Point Road DHVs. The additional tractor trailer truck trips as shown in Figures 24 and 25 for the I-526 mainline and Long Point Road, respectively, reflecting more growth at WWT than from the TDM, were added to the "base" DHVs in Figures 21 and 22. Figures 26, 27 and 28 display the 2030 DHV's including tractor trailer trucks for Seven Farms Drive, Long Point Road, and Hungryneck Boulevard interchanges, respectively.

# 2.3 2050 DHVs, Truck, Total

Traffic volumes in 2022 were grown with compounded growth rates based on the CHATs TDM to develop 2050 traffic volumes. **Figure 29** displays the 2050 DHV's for the Seven Farms Drive interchange and intersections. **Figure 30** displays the 2050 DHV's for the Long Point Road interchange and intersections. Growth of Wando Port will be incorporated into 2050 Long Point Road DHVs. The additional tractor trailer truck trips as shown in **Figures 31** and **32** for the I-526 mainline and Long Point Road, respectively, reflecting more growth at WWT than from the TDM, were added to the "base" DHVs in Figures 27 and 28. **Figures 33, 34 and 35** display the 2050 DHV volumes including tractor trailer trucks for Seven Farms Drive, Long Point Road, and Hungryneck Boulevard interchanges, respectively.

Figure 19: Seven Farms Dr. Interchange 2022 Design Hour Traffic Volumes (XX) PM Peak Intersection Volume Seven Farms Dr. Interchange 2022 Design Hour Traffic Volumes XX AM Peak Intersection Volume Ramp or Mainline Volume Interchange Name: Daniel Island 00 (00) Factored 2022 count used Intersection & Ramp Peak Hours Interchange Immediately East: Long Point Rd. Exit 28 Interchange Immediately West Clements Ferry Rd. Exit 23 Eastbound Exit Number: 24 Westbound Exit Number: a traine 3,901 (2,795) (60) (60) (4) 3,100 (3,935) 980 (940) (828) 585 15 912 (1,288) (294) 300 (318) 261 1,009 (582) 3,506 (2,430)

(XX) PM Peak Intersection Volume XX AM Peak Intersection Volume Ramp or Mainline Volume Long Point Rd. Interchange 2022 Design Hour Traffic Volumes Interchange Name: Long Point Rd. 00 (00) Factored 2022 count used Intersection & Ramp Peak Hours Interchange Immediately East: U.S. 17 N. Exit 29 U.S. 17 S. Exit 30 28 Interchange Immediately Westbound Exit Number: Eastbound Exit Number: Seven Farms Dr. Exit 24 Legend 5.5 060,1) (02,42) 2,783 (2,364) (991) 00 -5 DE (121) FT (95) 2,587 (3,386) 211 (45) 125 (45) 226 (45) हिंही हिंही 70 (ea) 300 (aa) Bene Pool Dr 1,966 (1,38) 70 (119) (TBI) 611-4 (873) 7 (95) (SUZ) 89E 7 (946) 669 --(327) 169 1,664) 1,068 (222) 83 (626) 812 (731,1) 505 - 10 - 10 495 (485) 279 (528) 4-1,442 (810) 4-672 (642) (91) 0/10 SF (05) 01 (91) 1,442 (619) L 727 (S15) (464) 469 S60 (967) è 901 ŧ 117 (4,628) (175) 717 200 200 200 488 (443) (87) 81 7 1,280 (1,520) 2000年 3,901 (2,795) (E) 51 J 0.3 Miles 3,100 (3,935) 1 000 mg 24 (106) 159 (55) 0,15 376 (360) 1 100 min 243 (132)

Figure 20: Long Point Rd. Interchange 2022 Design Hour Traffic Volumes

(XX) PM Peak Intersection Volume XX AM Peak Intersection Volume Ramp or Mainline Volume 2022 Peak Hour Traffic Volumes Ramp Peak Hours Interchange Immediately East: U.S. 17 Interchange Eailtbound Exit Number: U.M. 17 N. Exit 29 U.M. 17 S. Exit 30 Whistbound Exit Number: N.M. Interchange Immediately Long Point Rd. Exit 28 Interchange Name: U.S. 1,433 (996) 1,350 (1,368) 2,783 (2,364) 1,223 (1,580) 1,364 (1,806) 2,587 (3,386)

Figure 21: Hungryneck Blvd. Interchange 2022 Design Hour Traffic Volumes

(XX) PM Peak Intersection Volume Seven Farms Dr. Interchange 2030 Design Hour Traffic Volumes XX AM Peak Intersection Volume Ramp or Mainline Volume Interchange Name: Daniel Island Interchange Immediately East: Long Point Rd. Exit 28 24 Interchange Immediately W Clements Ferry Rd. Exit 23 Westbound Exit Number: Eastbound Exit Number: To state of the st F. O. CO. 4,429 (3,174) San Coas 1 3 6 5 5 CO 10 CO 3,519 (4,468) (16) 17 1,112 (1,067) 1,036 (1,463) 15 (330) in 1,146 (661) 3,981 (2,760)

Figure 22: Seven Farms Dr. Interchange 2030 Design Hour Traffic Volumes

(XX) PM Peak Intersection Volume XX AM Peak Intersection Volume Ramp or Mainline Volume Long Point Rd. Interchange 2030 Design Hour Traffic Volumes Interchange Name: Long Point Rd. Intersection & Ramp Peak Hours Interchange Immediately East: U.S. 17 N. Exit 29 U.S. 17 S. Exit 30 28 Westbound Exit Number: Interchange Immediately Eastbound Exit Number: Seven Farms Dr. Exit 24 Legend 45 (12,1) 087,1-(001) 50 -5 (138) 35 . - SI (19) 3,160 (2,684) 2,937 (3,844) 1,468) 847 (201) 991 (162) 125 (27) 05 2,209 (1,554 76 (129) 4- WA (502) 930 (991) 9 (19) (249) 93 + (800) 160 11041) 754 (104) 323 (25£,1) TQF 562 (551) 317 (599) 1,323(1,191 17 (22) 4-4,637 (030) u-963 (932) 190 (47) 1,637 (930) 11 [81] L Z87 (229) 1 1 L 1,618) 1,058 - 810 (SCE) P-181 1291 -12 4.815) 003 --CENT (878) 689 554 (503) (603) 700 T 50 (31) 1,453 (1,726) £ 165 (45) (201) 29-3 4,429 (3,174) **←** 0 (0) F 1 (5) (975) 500 T F-8(8) (601) [1 (0) 0 (9) 01 0.3 Miles 275 (150) (1. AA (17) (23) e 3,519 (4,468) -0 (0) 27 (1117) (a) 174 (60) 0,15 427 (409) 3 275 (150)

Figure 23: Long Point Rd. Interchange 2030 Design Hour Traffic Volumes

Figure 24: Seven Farms Dr. Interchange 2030 Tractor Trailer Trucks Only Volumes

